





# **Transportation Impact Study**

2438 Gottingen Street

October 4, 2018

## Submitted by:

Ekistics Plan + Design

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# 1. INTRODUCTION

This Transportation Impact Study follows HRM's Guidelines for the Preparation of Transportation Impact Studies (8<sup>th</sup> Edition) and general transportation engineering principles recommended for such studies. It is intended to address the transportation impacts that can be reasonably expected on the roadway and active transportation networks resulting from the:

• Addition of a multistory residential development as described below.

Transportation Impact
Studies are prepared to
ensure developments are
consistent with the
objectives and policies of the
Municipal Planning
Strategies / Municipal
Development Plans and the
Regional Plan

Proposed Development	2438 Gottingen Street, Halifax, Nova Scotia
Owner / Developer	Joseph Arab
Location	Between Gottingen Street and Creighton Street, and
Location	Between Charles Drive and Buddy Day Street.
Building	137 Residential Units in New Building
Bulluling	13 Units in Victoria Hall (existing)
Parking	Vehicles = 76 Indoor, Bicycle = 76 Class A, 15 Class B

# 2. EXISTING CONDITIONS

## 2.1 Study Area

The proposed development is located about 250 meters southeast of North Street (considered east for this study) in the block between Gottingen and Creighton Street, and between Charles Street and Buddy Daye Street. The development is in the middle of a larger residential area within a grid-based road network and only about 400 meters from the Halifax end of the Macdonald Bridge.

This area is heavily influenced by commuter traffic in the AM and PM peak hours which includes high volumes of traffic on Gottingen Street, and the frequent use of Creighton Avenue as a short cut route to bypass the queues that often occur when coming off the Macdonald Bridge and turning left onto Gottingen Street.

Creighton Street is a one-way street in the eastbound direction toward downtown and forms a one-way couplet with Maynard Street immediately to the south. Charles Street is a one-way street in the northbound direction towards Gottingen Street.

Figure 2-1: Study Area



## 2.2 Impacted Roadways

The following sections provide a brief summary of each of the key roadways in the study area.

Gottingen Street



Gottingen Street near the development is an urban three-lane roadway with one inbound lane and two outbound lanes complete with sidewalks on both sides of the road. There are regular midblock cross walks along the roadway and traffic is highly commuter oriented during the peak hours. As one of the primary corridors to downtown Halifax, the roadway also remains busy throughout the day with vehicular and pedestrian traffic. Roadside uses are a mix of low and medium rise residential units with some commercial / retail uses in some areas. Areas further east towards downtown become increasingly commercial.

## Creighton Street



Creighton Street is a local residential one-way street with sidewalks, numerous driveways and parking permitted on both sides of the street in many areas. Adjacent residential development includes predominantly low-rise multi-unit single family residences as well as some mid-rise buildings and institutional land uses including the Joseph Howe Elementary School at the intersection of Creighton and Charles Street.

## Charles Street



Charles Street on the west side of the development is a local residential one-way street running from Robie Street to Gottingen Street (two-way south of Robie). It has sidewalks on both sides of the roadway and the adjacent land uses include low and medium rise residential units with some commercial and small industrial properties near the development. Parking is permitted on both sides of the road in most areas with some short-term parking present adjacent to Joseph Howe School.

## Buddy Daye Drive



Buddy Daye Street east of the development is a two-way residential street with sidewalks and parking permitted on one side of the street in most areas. It is only 2 blocks in length between Maynard and Gottingen Street and the roadside includes low rise multi-unit residential buildings and auto some repair / autobody properties.

## 2.3 Active Transportation (AT)

The areas surrounding the development and the Halifax Peninsula in general are known for high levels of active transportation (AT) activity. In the area of the development, both local and commuter based AT traffic is expected including commuter traffic that users the Macdonald Bridge to cross the harbour. The development is well connected to surrounding areas through sidewalks and on-road facilities. All roadways in the area typically have sidewalks along both sides of the road and all major intersections provide pedestrian crossings across the intersection.

## **2.4** Vehicle Traffic

Recent and historical traffic counts were provided from HRM for all intersections and some road sections in the study area which were also supplemented by site observations during typical weekday peak traffic. The background counts reviewed in this study are provided in Appendix B of this report and the figures in Section 4 of this report show the network model incorporating the count volumes at key impacted intersections.

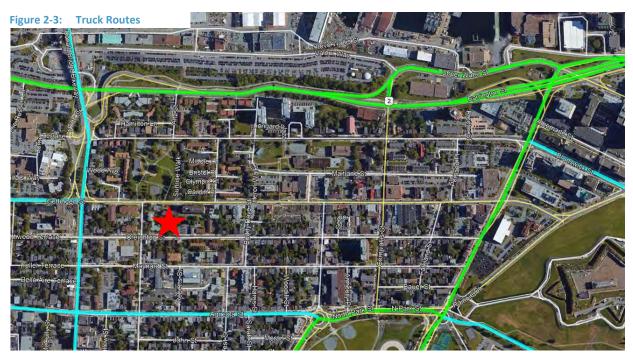
## 2.5 Transit

The development is the heart of one of Halifax's busiest transit areas which includes many routes on Gottingen and Barrington Street as well as North and Cornwallis Street. It is in close proximity to two major terminals with Scotia Square just over a kilometer to the east and the Bridge Terminal in Dartmouth about 2 kilometers to the north over the Macdonald Bridge.



## **2.6** Truck Routes

Halifax's By-Law T-400 "Respecting the Establishment of Truck Routes for Certain Trucking Motor Vehicles within the HRM" Barrington Street, Cogswell Street and North Park Street as "Full Time" truck routes. In addition, North Street, a portion of Gottingen Street west of North Street and Agricola Street are defined as daylight routes between the hours of 7 am and 9 pm. The red star identifies the location of the development near the middle of these routes which should allow for adequate access to the new development, though it is expected that delivery requirements will be minimal to this site.



# **3.** FUTURE CONDITIONS

## 3.1 Context

## 3.1.1 Analysis Time Horizon

Based on recommended HRM guidelines, the base year for this study has been established as 2018. Given the relatively low volumes predicted from the development and the high level of traffic dispersion that is expected, future traffic scenarios are not considered relevant to the results of this study.

## 3.1.2 Analysis Period

This area of Halifax is highly commuter oriented therefore, the weekday AM and PM peak hours are considered to be the critical periods for the analysis.

## 3.2 The Development

Future traffic related to the development is impacted only by the addition of the proposed development. It is possible that there may be minor modifications to existing buildings, but this is not expected to have any impact on transportation operations or safety performance. Construction of the new building will result in the removal of some existing parking, though it is assumed that this parking will relocate to the underground parkade in the future therefore no impact to traffic volumes was accounted for.

The new 16-story residential building includes 137 units and about 76 underground parking spaces with access to the underground parkade located off Creighton Street as shown in the figure below. This will be a right-in, right-out driveway due to Creighton Street being one-way in the eastbound direction.



## 3.3 Trip Generation

New traffic generated by the development was based on the Institute of Transportation Engineers (ITE) Trip Generation Guide and was limited to residential trips as no commercial or retail uses are proposed for the development. The existing Victoria Hall building will see a reduction of approximately 5 units, though for the purposes of this report it was assumed that trip from this portion of the development would remain the same as today. A more detailed summary of the trip generation rates and background calculations are provided in Appendix C of this report.

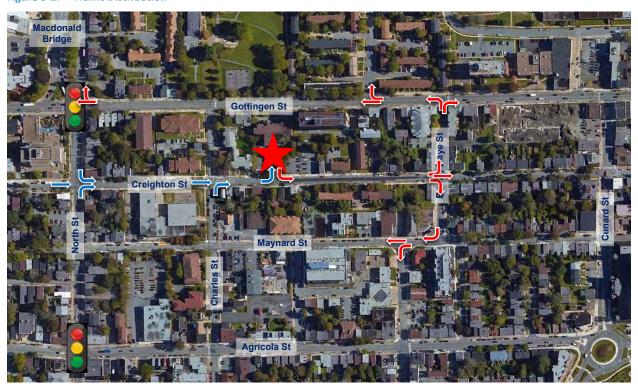
**Table 3-1: Trip Generation Table** 

		ITE Land		AM Peak		PM Peak			
		Use Type	Enter	Exit	Total	Enter	Exit	Total	
Apartments	137 Units	ITE 222	11	31	42	34	22	56	
Sub-Total			11	31	42	34	22	56	

## 3.4 Trip Distribution and Assignment

All traffic entering the site will do so as an eastbound left turn from Creighton Street and all exiting traffic will make a southbound left turn onto Creighton Street. Upstream and downstream of these movements, there are numerous route options that drivers can select depending on their origins and destinations as shown in the figure below. Note that only the movements immediately upstream and downstream are indicated and beyond these intersections are a wide variety of other options.

Figure 3-2: Traffic Distribution



# 4. ANALYSIS

## 4.1 Transportation Modelling

A microscopic traffic model was prepared using the Synchro/SimTraffic platform for the AM and PM peak hours of analysis for an isolated area surrounding the development. Limited formal analysis was carried out for this project as intersections surrounding the development operate at a high level of service presently, and with the development added to the road network. The following two figures show the turning movement volumes (grey boxes with black numbers) and overall intersection capacity utilization (ICU) percentages (blue boxes with black numbers) surrounding the development. The figures are for the AM and PM peak hours and for the purposes of this analysis, all volumes were increased by 20% from the counted volumes to account for any variations in traffic.

Figure 4-1: AM Peak Hour – Development Driveway, Creighton and Gottingen Street



Figure 4-2: PM Peak Hour – Development Driveway, Creighton and Gottingen Street



The figures suggest that there is significant excess capacity at all intersections along Creighton Drive and that drivers to and from the development are expected to experience very little delay or queuing. Beyond these intersections, traffic is dispersed significantly in different directions and therefore has essential no impact on existing traffic operations or safety performance.

# 5. Conclusions

This report analyzed the impacts of the addition of a new multi-story residential development proposed for 2438 Gottingen Street. The primary vehicle driveway to the underground parkade is located off Creighton Street which is a low volume, one-way residential street towards downtown Halifax. All movements to the development will be right-in and right-out due to Creighton being a one-way street, therefore the entrance and exit driveway can be accommodated with single lanes.

There are a wide variety of route options to get to and from the site resulting in significant traffic dispersion throughout the road network. Combined with the relatively low volumes destined to and from the development, there is very minimal impact anywhere throughout the adjacent road network. While the area can be busy during peak hour traffic, and Creighton can frequently be used as an alternate route to downtown, there is significant available excess capacity to accommodate the proposed development without any infrastructure upgrade requirements.

Both transit and active transportation modal shares are expected to be high in this area and it is likely that residents of this development will take advantage of these travel modes to some degree. In general, the development is highly compatible with surrounding land uses and is a desirable infilling location to help support regional planning initiatives.

We trust that this report satisfies the Halifax requirements for the preparation of Transportation Impact Studies for such a development. Should there be any questions or comments regarding the content of the study, please do not hesitate to contact the undersigned.

Sincerely,

Roger N. Boychuk, P.Eng. Senior Transportation Eningeer

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# **APPENDIX A**

## **Site Statistics**

#### 2018.09.20 2438 Gottingen St Project Summary

### **New Building**

Building Floor Level	GFA	Units	Parking Stalls	Bicycle Parking (Class A)	Bicycle Parking (Class B)
Parking -02			40		
Parking -01			36		
Main Level	11,340	6		62	15
Level 02	11,390	11			
Level 03	11,390	11			
Level 04	8,190	10			
Level 05	8,515	9			
Level 06	8,515	9			
Level 07	8,515	9			
Level 08	8,515	9			
Level 09	8,515	9			
Level 10	8,515	9			
Level 11	8,515	9			
Level 12	8,515	9			
Level 13	8,515	9			
Level 14	8,515	9			
Level 15	7,625	4			
Level 16	7,750	5			
Totals	142835	137	76	76	15

Level	Bachelor	1 Bed	2 Bed	Total Units
Main Level	0	4	2	6
Level 02	0	7	4	11
Level 03	0	7	4	11
Level 04	2	5	3	10
Level 05	1	4	4	9
Level 06	1	4	4	9
Level 07	1	4	4	9
Level 08	1	4	4	9
Level 09	1	4	4	9
Level 10	1	4	4	9
Level 11	1	4	4	9
Level 12	1	4	4	9
Level 13	1	4	4	9
Level 14	1	4	4	9
Level 15	0	0	4	4
Level 16	0	0	5	5
Total Units	12	63	62	137

Unit %	Bachelor	1 Bed	2 Bed	Total Units
	8.76	45.99	45.26	100

## Victoria Hall Building Floor Level

	GFA	Units	Parking Stalls
Main Level	6110	4	
Level 02	6110	4	
Level 03	6,110	5	
Totals	18330	13	

Total Units	150

Development Lot Area	
PID	00148791
Total Lot Area	36,100
Total Development GFA excluding Parkin	161165
Floor Area Ratio	4.46

# **APPENDIX B**

## **Traffic Counts**

CODE NO.

16-TM-349

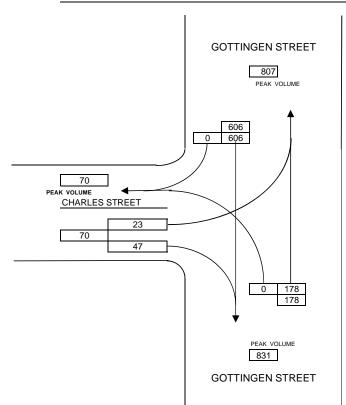
## MANUAL TRAFFIC COUNTS

INTERSECTION:		CHARLES STREET AT GOTTINGEN STREET											
	<u> </u>			0						WEATHE	R	C	_EAR
DAY DATE	MONTH	YEAR								RECOR	DER		AD
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OTDEET	STREET: CHARLES STREET GOTTINGEN STREET GOTTINGEN STREET												1
STREET: TIME:	FDC	M THE E	ACT	FROM THE WEST			FROM THE NORTH			FROM THE SOUTH			TOTAL
11ME: 15 MIN INTERVALS	I	S	R	I	S	R R	I	S	R	I	S	R	TOTAL
07:00:00 AM 07:15:00 AM	0	0	0	5	0	11	0	134	0	0	27	0	177
07:15:00 AM 07:30:00 AM	0	0	0	6	0	8	0	152	0	0	48	0	214
07:30:00 AM 07:45:00 AM	0	0	0	9	0	18	0	174	0	0	59	0	260
07:45:00 AM	0	0	0	3	0	10	0	146	0	0	44	0	203
07:45:00 AM   08:00:00 AM	U	U	U	3	U	10	U	146	U	U	44	U	203
TOTAL	0	0	0	23	0	47	0	606	0	0	178	0	854
PEAK		0			70			606			178		
15 MIN PEAK	EAK 0				108			696			236		
PEAK HOUR FACTOR 0					0.65		0.87			0.75			
TWO WAY TOTALS 0					70			807			831		FACTOR
							ı			1			1.03
											880		
DAY DATE	MONTH		Ī										
TUES 9	AUG	2016											
TIME:	FRC	M THE E	AST	FRC	OM THE V	VEST	FRO	M THE NO	RTH	FRO	M THE SC	OUTH	TOTAL
15 MIN INTERVALS	L	S	R	L	S	R	L	S	R	L	S	R	
08:00:00 AM 08:15:00 AM	0	0	0	5	0	12	0	187	0	0	49	0	253
08:15:00 AM 08:30:00 AM	0	0	0	5	0	7	0	166	0	0	51	0	229
08:30:00 AM 08:45:00 AM	0	0	0	3	0	6	0	161	0	0	55	0	225
08:45:00 AM 09:00:00 AM	0	0	0	9	0	3	0	124	0	0	52	0	188
		U	U		U								
TOTAL	0	0	0	22	0	28	0	638	0	0	207	0	895
PEAK		0			50			638		207			
15 MIN PEAK		0			68			748			220		
PEAK HOUR FACTOR		0			0.74			0.85			0.94		
TWO WAY TOTALS		0			50			867			873		
										•			FACTOR 1.03
													922

8/26/16 10:06 AM Record

INTERSECTION:

CHARLES STREET AT GOTTINGEN STREET

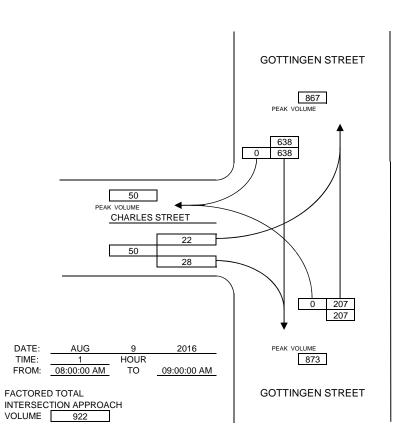


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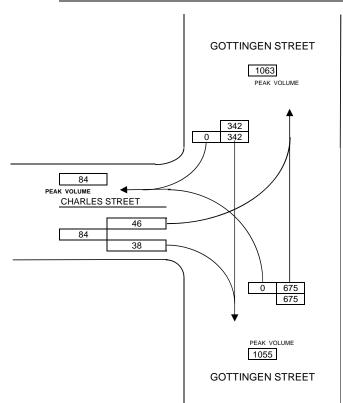
## MANUAL TRAFFIC COUNTS

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04:15:00 PM 04:30:00 PM	0	0	0	12	0	14	0	85	0	0	158	0	269
04:30:00 PM 04:45:00 PM	0	0	0	6	0	9	0	80	0	0	153	0	248
04:45:00 PM 05:00:00 PM	0	0	0	13	0	8	0	85	0	0	182	0	288
·													
TOTAL	0	0	0	46	0	38	0	342	0	0	675	0	1101
PEAK		0		84				342			675		
15 MIN PEAK					104		368			728			
PEAK HOUR FACTOR		0		0.81			0.93			0.93			
TWO WAY TOTALS		0			84			1063			1055		FACTOR
													1.03
DAY DATE	MONITU	\/E											1134
DAY DATE TUES 9	MONTH AUG	2016	1										
1020	7100	2010											
TIME:	FRO	M THE E	AST	FROM THE WEST			FROM THE NORTH			FROM THE SOUTH			TOTAL
15 MIN INTERVALS	L	S	R	L	S	R	L	S	R	L	S	R	
05:00:00 PM 05:15:00 PM	0	0	0	7	0	7	0	92	0	0	128	0	234
05:15:00 PM 05:30:00 PM	0	0	0	13	0	16	0	113	0	0	145	0	287
05:30:00 PM 05:45:00 PM	0	0	0	4	0	9	0	81	0	0	87	0	181
05:45:00 PM 06:00:00 PM	0	0	0	4	0	5	0	97	0	0	70	0	176
				ı						1	ı		
TOTAL	0	0	0	28	0	37	0	383	0	0	430	0	878
PEAK	0				65			383			430		
15 MIN PEAK	0			116			452			580			
PEAK HOUR FACTOR	R FACTOR 0			0.56			0.85			0.74			
TWO WAY TOTALS		0			65			841			850		
													1.03
													904

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INTERSECTION:

CHARLES STREET AT GOTTINGEN STREET

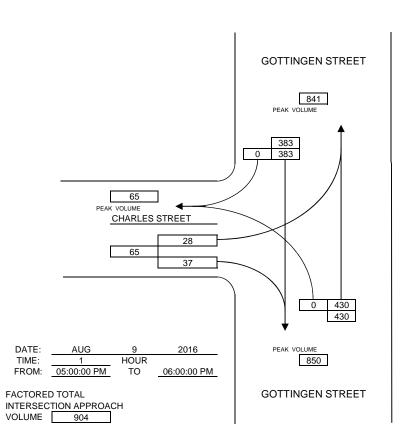


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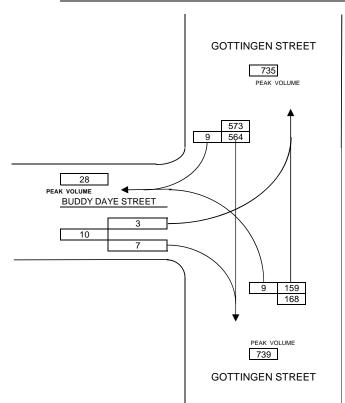
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THURS 11	AUG	2016											
	r-												•
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TIME:		M THE E		FROM THE WEST			FROM THE NORTH			FROM THE SOUTH L S R			TOTAL
15 MIN INTERVALS	L	S	R	L	S	R	L	S	R	_		R	4=0
07:00:00 AM 07:15:00 AM	0	0	0	1	0	1	0	134	0	1 -	41	0	178
07:15:00 AM 07:30:00 AM	0	0	0	0	0	1	0	119	2	5	30	0	157
07:30:00 AM 07:45:00 AM	0	0	0	0	0	1	0	151	4	2	40	0	198
07:45:00 AM   08:00:00 AM	0	0	0	2	0	4	0	160	3	1	48	0	218
TOTAL	0	0	0	3	0	7	0	564	9	9	159	0	751
PEAK	0	0	U	3	10	,	0	573	3	3	168	0	731
15 MIN PEAK		0			24		652			196			
PEAK HOUR FACTOR		0		0.42			0.88			0.86			
TWO WAY TOTALS		0			28		735				739		FACTOR
TWO WAY TOTALS		U			28			735			739		1.02
										766			
DAY DATE	MONTH	YEAR											
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													TOTAL
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08:00:00 AM 08:15:00 AM	0	0	0	1	0	2	0	155	7	2	45	0	212
08:15:00 AM 08:30:00 AM	0	0	0	3	0	3	0	170	7	1	49	0	233
08:30:00 AM 08:45:00 AM	0	0	0	2	0	4	0	179	4	2	45	0	236
08:45:00 AM   09:00:00 AM	0	0	0	4	0	5	0	146	3	5	48	0	211
TOTAL		0	0	40	_	14	0	050	21	40	187	0	892
TOTAL	0	0	U	10	0	14	U	650	21	10		U	892
PEAK		0			24			671		197			
15 MIN PEAK					36		732			212			
PEAK HOUR FACTOR		0		0.67			0.92			0.93			
TWO WAY TOTALS		0			55 868				861			FACTOR	
													1.02 910
													910

8/26/16 11:26 AM Record

INTERSECTION:

BUDDY DAYE STREET AT GOTTINGEN STREET

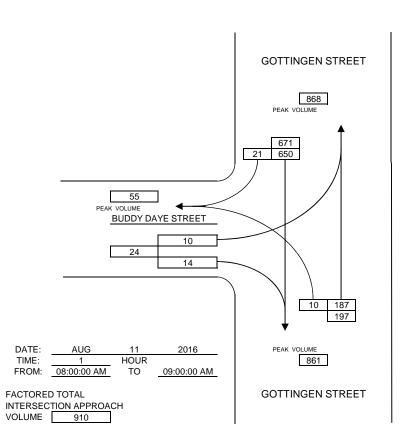


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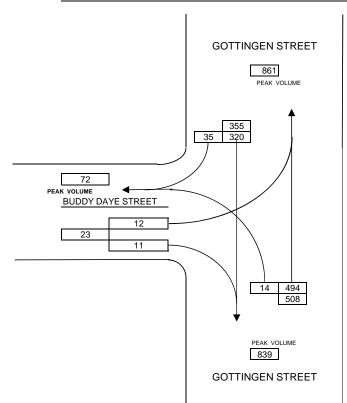
## MANUAL TRAFFIC COUNTS

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WILKOLO HOW.				DODDII	<i>&gt;</i> /(12 011	KEET /KT C		LITOTICE		WEATHE	ER.	PARTI '	Y CLOUDY
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15 MIN INTERVALS	L	S	R	L	S	R	L	S	R	L	S	R	
04:00:00 PM 04:15:00 PM	0	0	0	2	0	3	0	66	10	4	115	0	200
04:15:00 PM 04:30:00 PM	0	0	0	5	0	5	0	92	8	2	131	0	243
04:30:00 PM 04:45:00 PM	0	0	0	1	0	2	0	90	7	4	138	0	242
04:45:00 PM 05:00:00 PM	0	0	0	4	0	1	0	72	10	4	110	0	201
TOTAL	0	0	0	12	0	11	0	320	35	14	494	0	886
PEAK		0			23			355			508		
15 MIN PEAK 0 40 400 568 PEAK HOUR FACTOR 0 0.58 0.89 0.89													
PEAK HOUR FACTOR         0         0.58         0.89         0.89           TWO WAY TOTALS         0         72         861         839													
TWO WAY TOTALS		0			72			861			839		FACTOR
				1						1			1.02
													904
DAY DATE	MONTH		•1										
THURS 11	AUG	2016											
TIME.	FDC	NATUE E	A O.T.	ED.	NA THE V	VECT	FDO	NA TUE NO	DTU	- FDO	MATUE OC	NITII	TOTAL
TIME: 15 MIN INTERVALS	L	M THE E S	ASI R	L	OM THE V S	VEST R	L L	M THE NO S	RIH R	L	M THE SC S	R R	TOTAL
05:00:00 PM 05:15:00 PM	0	0	0	4	0	1	0	75	6	5	127	0	218
05:15:00 PM	<u> </u>	0	0	1	0	3	0	70	13	4	71	0	162
05:30:00 PM 05:45:00 PM	0	0	0	4	0	4	0	78	7	2	79	0	174
05:45:00 PM	+	0	0	1	0	4	0	72	9	6	71	0	163
05.45.00 FM   06.00.00 FM	U	U	U		U	4	U	12	9	0	/ 1	U	103
TOTAL	0	0	0	10	0	12	0	295	35	17	348	0	717
PEAK		0			22	ı		330			365		
15 MIN PEAK		0			32			340			528		
PEAK HOUR FACTOR		0			0.69			0.97			0.69		
TWO WAY TOTALS		0			74			688			672		FACTOR
				1	17			000		I	012		1.02
													731

8/26/16 11:31 AM Record

INTERSECTION:

BUDDY DAYE STREET AT GOTTINGEN STREET

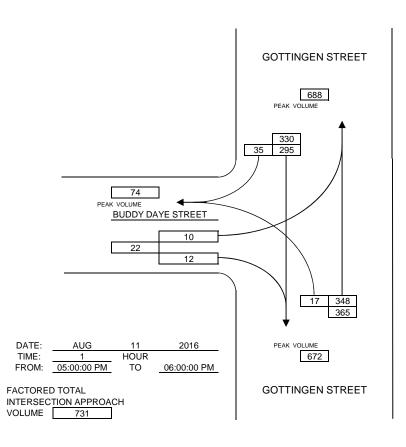


 DATE:
 AUG
 11
 2016

 TIME:
 1
 HOUR

 FROM:
 04:00:00 PM
 TO
 05:00:00 PM

FACTORED TOTAL
INTERSECTION APPROACH
VOLUME 904



8/26/16 11:31 AM Graphic

CODE NO.

18RQ236

## MANUAL TRAFFIC COUNTS

INTERSECTIO	NI-			DEICUT	ON CTDE	ET AT NO	DTU CT	REET & NO	DTUMOO	D TEDDA	CF.		1	
INTERSECTIO	IN.			KEIGHI	JNSIKE	ELALING	KIRSII	KEET & INC	KINWOO	DIEKKA	WEATH	FR	SI	JNNY
DAY	DATE	MONTH	YEAR								RECOR			BRADLEY
TUES.	31	JULY	2018	Ī										
				L										
STREET:		NO	RTH STR	EET	NO	RTH STR	EET	NORTH	WOOD TE	RRACE	CREIC	SHTON S	TREET	
TIME:		FRC	M THE E	AST	FRC	M THE V	VEST	FRO	M THE NO	RTH	FRO	M THE S	OUTH	TOTAL
15 MIN INTER		L	S	R	L	S	R	L	S	R	L	S	R	
07:02:00 AM	07:17:00 AM	26	56	0	0	71	5	3	2	2	0	0	0	165
07:17:00 AM	07:32:00 AM	26	64	0	0	95	7	0	3	4	0	0	0	199
07:32:00 AM	07:47:00 AM	39	78	0	0	76	5	0	2	4	0	0	0	204
07:47:00 AM	08:02:00 AM	32	93	0	0	85	4	2	2	8	0	0	0	226
		_												
TOTAL		123	291	0	0	327	21	5	9	18	0	0	0	794
PEAK			414			348			32			0		
4(15 MIN PEA			500			408			48			0		
PEAK HOUR F			0.83 746			0.85 657			0.67 32			0		AAWT
TWO WAY TO	IALS		746			657			32			153		FACTOR 1.03
DAY	DATE	MONTH	VEAD											818
TUES.	31			ī										
TUES.	31	JULY	2018	l										
TIME:		EDC	OM THE E	AST	FRC	M THE V	VEST	FPO	M THE NO	PTH	FPO	M THE S	OLITH	TOTAL
15 MIN INTER	V/AI S	I	S	R	I	S	R	I	S	R	I	S	R	IOIAL
13 MIN HATEIC	VALO			- 11			- 1			- 11			- 11	

TIME:		FRO	OM THE E	AST	FRC	M THE V	VEST	FRO	M THE NO	RTH	FRO	M THE S	OUTH	TOTAL
15 MIN INTERV	/ALS	L	s	R	L	s	R	L	S	R	L	S	R	
08:02:00 AM	08:17:00 AM	26	77	0	0	82	1	3	1	2	0	0	0	192
08:17:00 AM	08:32:00 AM	25	73	0	0	94	3	2	2	7	0	0	0	206
08:32:00 AM	08:47:00 AM	27	98	0	0	77	2	0	5	7	0	0	0	216
08:47:00 AM	09:02:00 AM	15	82	0	0	85	9	0	6	6	0	0	0	203
		•	•	•	•	•		•	•	•	•	•		
TOTAL		93	330	0	0	338	15	5	14	22	0	0	0	817
PEAK	EAK		423			353			41			0		
4(15 MIN PEAK	15 MIN PEAK)		500			388			48			0		
						0.04						_		

#### Intersection Peak Hour

		NO	RTH STR	EET	NOI	RTH STR	EET	NORTH	WOOD TER	RRACE	CREIG	HTON ST	REET	Total
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Total
	Car	110	332	3	1	328	11	6	9	24	0	0	0	824
7:45 - 8:45	Truck	0	9	0	0	9	0	1	1	0	0	0	0	20
	Bicycle	0	2	0	1	17	0	0	10	0	0	1	0	31
	Vehicle Total	110	343	3	2	354	11	7	20	24	0	1	0	875
	Approach Factor		0.9			0.9			0.91			0.25		FACTOR
														1
														875

#### Peak Hour Pedestrians

			NE			NW			SW			SE		Total
7:45 - 8:45		Left	Right	Total	I Otal									
	Pedestrians	5	10	15	24	3	27	1	14	15	11	7	18	75

#### Car traffic

Interval starts	NO	RTH STR	EET	NOI	RTH STR	EET	NORTH	WOOD TER	RACE	CREIG	HTON ST	REET	Total
interval starts	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Total
7:02	26	55	0	0	69	5	3	2	2	0	0	0	163
7:15	26	60	0	0	93	6	0	3	4	0	0	0	193
7:30	39	75	0	0	72	4	0	2	4	0	0	0	196
7:45	32	91	0	0	82	4	2	2	8	0	0	0	222
8:00	26	74	0	0	82	2	2	1	2	0	0	0	190
8:15	25	70	0	0	88	3	2	2	7	0	0	0	197
8:30	27	97	0	0	76	2	0	4	7	0	0	0	215
8:45	15	80	0	0	82	8	0	6	6	0	0	0	197
TOTAL	216	602	0	0	644	34	9	22	40	0	0	0	1573

#### Truck traffic

Interval starts	NO	RTH STR	EET	NO	RTH STR	EET	NORTH	WOOD TER	RACE	CREIG	HTON ST	REET	Total
iiilei vai StaitS	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	iotai
7:02	0	1	0	0	2	0	0	0	0	0	0	0	3
7:15	0	4	0	0	2	1	0	0	0	0	0	0	7
7:30	0	3	0	0	4	1	0	0	0	0	0	0	8
7:45	0	2	0	0	3	0	0	0	0	0	0	0	5
8:00	0	3	0	0	2	0	1	0	0	0	0	0	6
8:15	0	3	0	0	2	0	0	0	0	0	0	0	5
8:30	0	1	0	0	2	0	0	1	0	0	0	0	4
8:45	0	2	0	0	1	0	0	0	0	0	0	0	3
TOTAL	0	40	Δ.	0	40	2	4	4	0	٥	0	٥	44

## Bicycle traffic

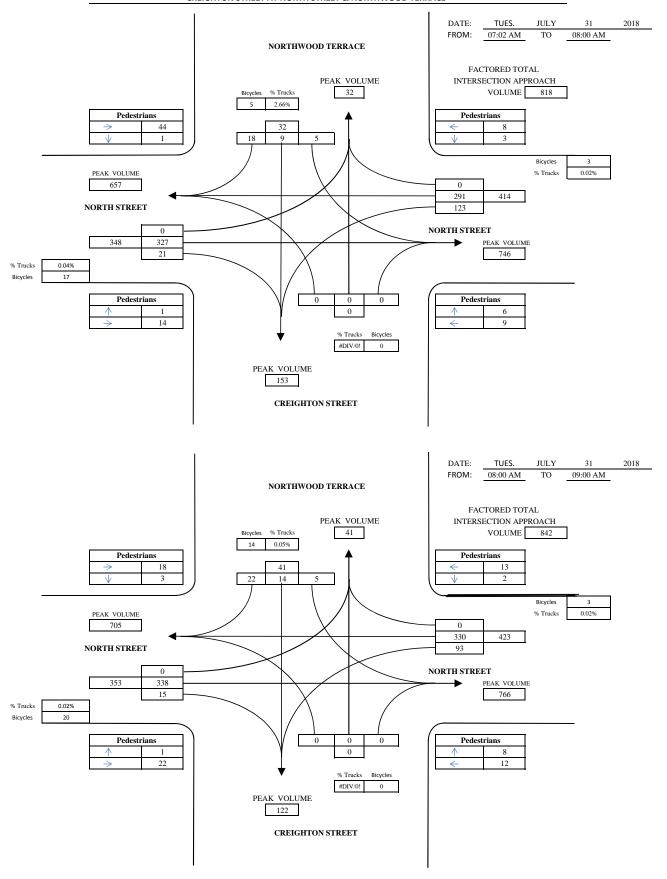
Interval atoms	NO	RTH STR	EET	NOI	RTH STR	EET	NORTH	WOOD TER	RACE	CREIG	HTON ST	REET	Total
Interval starts	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Total
7:02	0	1	0	0	5	1	0	0	1	0	0	0	8
7:15	0	1	0	0	4	0	0	0	0	0	0	0	5
7:30	0	1	0	0	6	0	1	3	0	0	0	0	11
7:45	0	0	0	0	1	0	0	0	0	0	0	0	1
8:00	0	1	0	0	3	0	0	6	0	0	0	0	10
8:15	0	0	0	0	8	0	0	2	0	0	0	0	12
8:30	0	1	0	0	5	0	0	2	0	0	0	0	8
8:45	0	1	0	0	3	1	1	2	1	0	0	0	10
ΤΟΤΔΙ	Ω	6	0	0	35	2	2	15	2	0	0	0	65

#### Pedestrian volumes

Interval starts		NE			NW			SW			SE		Total
interval starts	Left	Right	Total	Total									
7:02	0	2	2	15	0	15	1	4	5	3	1	4	26
7:15	0	0	0	15	0	15	0	3	3	3	3	6	24
7:30	0	4	4	5	1	6	0	4	4	1	1	2	16
7:45	3	2	5	9	0	9	0	3	3	2	1	3	20
8:00	0	2	2	6	0	6	0	1	1	1	2	3	12
8:15	0	0	0	7	1	8	0	5	5	2	4	6	19
8:30	2	6	8	2	2	4	1	5	6	6	0	6	24
8:45	0	5	5	3	0	3	0	11	11	3	2	5	24
TOTAL	5	21	26	62	4	66	2	36	38	21	14	35	165

### VEHICULAR GRAPHIC SUMMARY SHEET

#### **CREIGHTON STREET AT NORTH STREET & NORTHWOOD TERRACE**



CODE NO.

18RQ236

## MANUAL TRAFFIC COUNTS

INTERSECTIO	N:		С	REIGHT	ON STRE	ET AT NO	ORTH ST	REET & NO	RTHWOOI	) TERRA	CE		1	
		,									WEATH	ER	SU	JNNY
DAY	DATE	MONTH	YEAR								RECOR	DER	LIAM E	BRADLEY
TUES.	31	JULY	2018											
					,			,						_
STREET:		NOI	RTH STR	EET	NO	RTH STR	EET	NORTH	WOOD TE	RRACE	CREIC	SHTON S	TREET	
TIME:		FRC	OM THE E	AST	FRO	OM THE V	/EST	FRO	M THE NO	RTH	FRO	M THE S	OUTH	TOTAL
15 MIN INTER	RVALS	L	S	R	L	S	R	L	S	R	L	S	R	
04:00:00 PM	04:15:00 PM	3	91	0	0	155	1	9	4	13	0	0	0	276
04:15:00 PM	04:30:00 PM	15	104	0	0	162	8	0	2	6	0	0	0	297
04:30:00 PM	04:45:00 PM	17	98	0	0	136	1	1	5	6	0	0	0	264
04:45:00 PM	05:00:00 PM	10	103	0	0	167	7	2	2	6	0	0	0	297
TOTAL		45	396	0	0	620	17	12	13	31	0	0	0	1134
PEAK			441			637			56			0		
4(15 MIN PEA	K)		476			696			104			0		
PEAK HOUR I	FACTOR		0.93			0.92			0.54			0		AAWT
TWO WAY TO	TALS		1073			1064			56			75		FACTOR
														1.03
														1168
DAY	DATE	MONTH	YFAR											•

	DAY	DATE	MONTH	YEAR
ı	THES	31	JULY	2018

TIME:		FRO	OM THE B	AST	FRC	OM THE V	/EST	FROI	M THE NO	RTH	FRO	M THE S	OUTH	TOTAL
15 MIN INTER	VALS	L	S	R	L	S	R	L	S	R	L	S	R	
05:00:00 PM	05:15:00 PM	7	109	0	0	122	7	2	0	- 1	0	0	0	248
05:15:00 PM	05:30:00 PM	- 11	108	0	0	113	6	3	2	5	0	0	0	248
05:30:00 PM	05:45:00 PM	9	79	0	0	104	4	0	2	3	0	0	0	201
05:45:00 PM	06:00:00 PM	4	96	0	0	97	4	1	0	2	0	0	0	204
TOTAL		31	392	0	0	436	21	6	4	11	0	0	0	901
PEAK			423	•		457	•		21	•		0	•	
4(15 MIN PEAR	<)		476			516			40			0		
PEAK HOUR F	ACTOR		0.89			0.89			0.53			0		AAWT
TWO WAY TOT	TALS		865			860			21			56		FACTOR
														1.03
														000

#### Intersection Peak Hour

		NO	RTH STR	EET	NOI	RTH STR	EET	NORTH	WOOD TER	RRACE	CREIG	HTON S	TREET	Total
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Total
	Car	45	384	1	1	611	16	12	12	31	0	0	0	1113
16:00-17:00	Truck	0	11	0	0	8	1	0	1	0	0	0	0	21
	Bicycle	0	11	0	0	11	0	1	4	1	0	0	1	29
	Vehicle Total	45	406	1	1	630	17	13	17	32	0	0	1	1163
	Approach Factor		0.94			0.91			0.57			0.25		FACTOR
														1
											1163			

#### **Peak Hour Pedestrians**

			NE			NW			SW			SE		Total
16:00-17:00		Left	Right	Total	I Otal									
	Pedestrians	7	37	44	7	2	σ	3	47	50	17	5	22	125

#### Car traffic

Interval starts	NO	RTH STR	EET	NOI	RTH STR	EET	NORTH	WOOD TER	RRACE	CREIG	SHTON S	TREET	Total
iiitei vai starts	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	IUIAI
16:00	3	88	0	0	152	1	9	4	13	0	0	0	270
16:15	15	102	0	0	160	7	0	2	6	0	0	0	292
16:30	17	95	0	0	135	1	1	4	6	0	0	0	259
16:45	10	100	0	0	165	7	2	2	6	0	0	0	292
17:00	7	106	0	0	120	7	2	0	1	0	0	0	243
17:15	11	106	0	0	114	6	3	2	5	0	0	0	247
17:30	9	75	0	0	102	4	0	2	3	0	0	0	195
17:45	4	94	0	0	93	4	1	0	2	0	0	0	198
TOTAL	76	766	0	0	1041	37	18	16	42	0	0	0	1996

#### Truck traffic

Interval starts	NO	RTH STR	EET	NO	RTH STR	EET	NORTH	WOOD TER	RACE	CREIG	SHTON S	TREET	Total
ilitervai starts	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	I Otal
16:00	0	3	0	0	3	0	0	0	0	0	0	0	6
16:15	0	2	0	0	2	1	0	0	0	0	0	0	5
16:30	0	3	0	0	1	0	0	1	0	0	0	0	5
16:45	0	3	0	0	2	0	0	0	0	0	0	0	5
17:00	0	3	0	0	1	0	0	0	0	0	0	0	4
17:15	0	2	0	0	2	0	0	0	0	0	0	0	4
17:30	0	4	0	0	3	0	0	0	0	0	0	0	7
17:45	0	2	0	0	4	0	0	0	0	0	0	0	6
TOTAL	Λ	22	0	٥	10	- 1	۸	- 1	0	٥	0	0	42

## Bicycle traffic

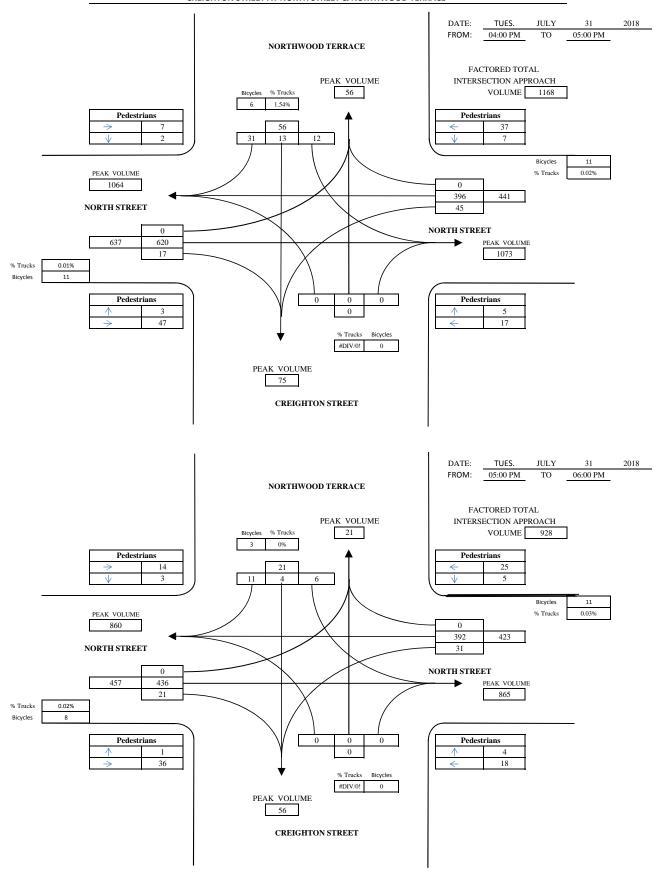
Interval starts	NO	RTH STR	EET	NOI	RTH STR	EET	NORTH	WOOD TER	RRACE	CREIG	SHTON S	TREET	Total
Interval starts	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Total
16:00	0	3	0	0	1	0	0	1	0	0	0	0	5
16:15	0	1	0	0	3	0	0	0	0	0	0	0	4
16:30	0	4	0	0	3	0	1	1	1	0	0	0	11
16:45	0	3	0	0	4	0	0	2	0	0	0	0	9
17:00	0	2	0	0	2	0	0	0	0	0	0	0	4
17:15	0	3	0	0	0	0	0	0	0	0	0	0	3
17:30	0	5	0	0	2	0	0	1	0	0	0	0	8
17:45	0	1	0	0	4	0	0	2	0	0	0	0	7
ΤΟΤΔΙ	Ω	22	Ω	0	19	Ω	1	7	1	0	0	0	51

#### Pedestrian volumes

Interval starts		NE			NW			SW			SE		Total
interval starts	Left	Right	Total	Iotai									
16:00	2	10	12	2	2	4	1	11	12	7	0	7	35
16:15	1	12	13	1	0	1	0	11	11	1	2	3	28
16:30	2	7	9	1	0	1	1	17	18	3	3	6	34
16:45	2	8	10	3	0	3	1	8	9	6	0	6	2
17:00	1	4	5	3	1	4	0	18	18	1	1	2	2
17:15	1	5	9	1	1	2	1	6	7	7	0	7	2:
17:30	0	8	8	4	1	5	0	10	10	8	2	10	33
17:45	3	8	11	6	0	6	0	2	2	2	1	3	2:
TOTAL	12	62	74	21	5	26	4	83	87	35	9	44	23

### VEHICULAR GRAPHIC SUMMARY SHEET

#### **CREIGHTON STREET AT NORTH STREET & NORTHWOOD TERRACE**



## **HOURLY TRAFFIC COUNTER SUMMARY**

HALIFAX REGIONAL MUNICIPALITY
TRANSPORTATION AND PUBLIC WORKS
TRAFFIC AND RIGHT OF WAY

PM

CODE No. 18-VOL-183

COUNTER No. Houston Radars 0007/908

File Name: R:\TPW\Engineering\Trafl

Region: WESTERN

## **HOURLY TRAFFIC COUNTER SUMMARY**

**DISTRICT** WESTERN

**CODE No.** 18-VOL-183

**COUNTER No. HOUSTON RADAR 0007** 

HALIFAX REGIONAL MUNICIPALITY
TRANSPORTATION AND PUBLIC WORKS
TRAFFIC AND RIGHT OF WAY

PΜ

## **HOURLY TRAFFIC COUNTER SUMMARY**

**DISTRICT** WESTERN

HALIFAX REGIONAL MUNICIPALITY TRANSPORTATION AND PUBLIC WORKS

PΜ

# **APPENDIX C**

# **Trip Generation**

## **Trip Generation Summary**

Alternative: Alternative 1

Phase:

Project: 2438 Gottingen Street

Open Date: 9/27/2018

Analysis Date: 9/28/2018

	V	/eekday A Adjacent			V	Veekday P Adjacent				We	ekday			Sa	turday	
ITE Land Use	*	Enter	Exit	Total	*	Enter	Exit	Total	*	Enter	Exit	Total	*	Enter	Exit	Total
222 Apartments 137 Dwelling Units		11	31	42		34	22	56		362	361	723		337	337	674
Unadjusted Volume		11	31	42		34	22	56		362	361	723		337	337	674
Internal Capture Trips		0	0	0		0	0	0		0	0	0		0	0	0
Pass-By Trips		0	0	0		0	0	0		0	0	0		0	0	0
Volume Added to Adjacent Streets		11	31	42		34	22	56		362	361	723		337	337	674

Total Weekday AM Peak Hour of Adjacent Street Traffic Internal Capture = 0 Percent

Total Weekday PM Peak Hour of Adjacent Street Traffic Internal Capture = 0 Percent

Total Weekday Internal Capture = 0 Percent

Total Saturday Internal Capture = 0 Percent

\* - Custom rate used for selected time period.

# **APPENDIX D**

# **Synchro Output**

	۶	<b>→</b>	•	1	<b>←</b>	•	4	<b>†</b>	1	-	ļ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	ĵ.						<b>1</b>			4	
Traffic Volume (veh/h)	5	15	22	0	0	0	0	338	16	100	330	0
Future Volume (Veh/h)	5	15	22	0	0	0	0	338	16	100	330	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	7	20	29	0	0	0	0	441	21	130	430	0
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	1142	1152	430	1180	1142	452	430			462		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1142	1152	430	1180	1142	452	430			462		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	96	89	95	100	100	100	100			88		
cM capacity (veh/h)	162	174	625	132	177	608	1129			1099		
Direction, Lane #	EB 1	EB 2	NB 1	SB 1								
Volume Total	7	49	462	560								
Volume Left	7	0	0	130								
Volume Right	0	29	21	0								
cSH	162	304	1700	1099								
Volume to Capacity	0.04	0.16	0.27	0.12								
Queue Length 95th (m)	1.0	4.3	0.0	3.0								
Control Delay (s)	28.3	19.1	0.0	3.1								
Lane LOS	D	C	0.0	A								
Approach Delay (s)	20.3	U	0.0	3.1								
Approach LOS	C		0.0	0.1								
Intersection Summary												
Average Delay			2.7									
Intersection Capacity Utilizat	tion		63.3%	IC	U Level o	of Service			В			
Analysis Period (min)	·		15									

	۶	<b>→</b>	*	•	<b>←</b>	•	1	<b>†</b>	~	1	<b>↓</b>	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	<b>^</b>						<b>^</b>	7			
Traffic Volume (veh/h)	25	106	0	0	0	0	0	25	12	0	0	0
Future Volume (Veh/h)	25	106	0	0	0	0	0	25	12	0	0	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	33	138	0	0	0	0	0	33	16	0	0	0
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	33	49	0	102	33	33	0			49		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	33	49	0	102	33	33	0			49		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	97	84	100	100	100	100	100			100		
cM capacity (veh/h)	974	843	1085	768	860	1041	1623			1558		
Direction, Lane #	EB 1	EB 2	NB 1	NB 2								
Volume Total	33	138	33	16								
Volume Left	33	0	0	0								
Volume Right	0	0	0	16								
cSH	974	843	1700	1700								
Volume to Capacity	0.03	0.16	0.02	0.01								
Queue Length 95th (m)	0.8	4.4	0.0	0.0								
Control Delay (s)	8.8	10.1	0.0	0.0								
Lane LOS	A	В	0.0	0.0								
Approach Delay (s)	9.9		0.0									
Approach LOS	A		0.0									
Intersection Summary												
Average Delay			7.7									
Intersection Capacity Utiliza	ition		16.7%	IC	U Level o	of Service			Α			
Analysis Period (min)			15	-								

	<b>→</b>	•	1	•	1	1
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>†</b>			<b>^</b>	*	7
Traffic Volume (veh/h)	638	0	0	207	22	28
Future Volume (Veh/h)	638	0	0	207	22	28
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	832	0	0	270	29	37
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume			832		967	832
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			832		967	832
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)					<b></b>	0.0
tF(s)			2.2		3.5	3.3
p0 queue free %			100		88	88
cM capacity (veh/h)			796		252	312
	ED 4	14/D 4		ND 4		V
Direction, Lane #	EB 1	WB 1	WB 2	NB 1	NB 2	
Volume Total	832	135	135	29	37	
Volume Left	0	0	0	29	0	
Volume Right	0	0	0	0	37	
cSH	1700	1700	1700	252	312	
Volume to Capacity	0.49	0.08	0.08	0.12	0.12	
Queue Length 95th (m)	0.0	0.0	0.0	2.9	3.0	
Control Delay (s)	0.0	0.0	0.0	21.1	18.1	
Lane LOS				С	С	
Approach Delay (s)	0.0	0.0		19.4		
Approach LOS				С		
Intersection Summary						
Average Delay			1.1			
Intersection Capacity Utiliza	ation		50.3%	IC	U Level c	f Service
Analysis Period (min)			15			

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Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	*	<b>^</b>			*	
Traffic Volume (veh/h)	11	107	0	0	31	0
Future Volume (Veh/h)	11	107	0	0	31	0
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	14	140	0	0	40	0
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	0				168	0
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	0				168	0
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	99				95	100
cM capacity (veh/h)	1623				815	1085
Direction, Lane #	EB 1	EB 2	SB 1			
Volume Total	14	140	40			
Volume Left	14	0	40			
Volume Right	0	0	0			
cSH	1623	1700	815			
Volume to Capacity	0.01	0.08	0.05			
Queue Length 95th (m)	0.2	0.0	1.2			
Control Delay (s)	7.2	0.0	9.6			
Lane LOS	Α		Α			
Approach Delay (s)	0.7		9.6			
Approach LOS			Α			
Intersection Summary						
Average Delay			2.5			
Intersection Capacity Utiliz	zation		16.8%	IC	U Level c	of Service
Analysis Period (min)			15			
510 1 01104 (111111)						

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	ĵ»						ĵ.			ર્ન	
Traffic Volume (veh/h)	12	15	31	0	0	0	0	620	30	59	396	0
Future Volume (Veh/h)	12	15	31	0	0	0	0	620	30	59	396	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	16	20	40	0	0	0	0	809	39	77	517	0
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	1500	1519	517	1550	1500	828	517			848		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1500	1519	517	1550	1500	828	517			848		
tC, single (s)	*6.0	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	89	81	93	100	100	100	100			90		
cM capacity (veh/h)	148	107	558	68	110	371	1049			790		
Direction, Lane #	EB 1	EB 2	NB 1	SB 1								
Volume Total	16	60	848	594								
Volume Left	16	0	0	77								
Volume Right	0	40	39	0								
cSH	148	232	1700	790								
Volume to Capacity	0.11	0.26	0.50	0.10								
Queue Length 95th (m)	2.7	7.6	0.0	2.5								
Control Delay (s)	32.2	25.8	0.0	2.5								
Lane LOS	D	D		Α								
Approach Delay (s)	27.1		0.0	2.5								
Approach LOS	D											
Intersection Summary												
Average Delay			2.3									
Intersection Capacity Utiliza	ation		83.6%	IC	U Level	of Service			Е			
Analysis Period (min)			15									

\* User Entered Value

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	<b>^</b>						<b>†</b>	7			
Traffic Volume (veh/h)	40	64	0	0	0	0	0	44	15	0	0	0
Future Volume (Veh/h)	40	64	0	0	0	0	0	44	15	0	0	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	52	83	0	0	0	0	0	57	20	0	0	0
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	57	77	0	98	57	57	0			77		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	57	77	0	98	57	57	0			77		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)						<u> </u>						
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	94	90	100	100	100	100	100			100		
cM capacity (veh/h)	940	813	1085	814	834	1009	1623			1522		
						1000	1020			1022		
Direction, Lane #	EB 1	EB 2	NB 1	NB 2								
Volume Total	52	83	57	20								
Volume Left	52	0	0	0								
Volume Right	0	0	0	20								
cSH	940	813	1700	1700								
Volume to Capacity	0.06	0.10	0.03	0.01								
Queue Length 95th (m)	1.3	2.6	0.0	0.0								
Control Delay (s)	9.1	9.9	0.0	0.0								
Lane LOS	Α	Α										
Approach Delay (s)	9.6		0.0									
Approach LOS	Α											
Intersection Summary												
Average Delay 6.1		6.1										
Intersection Capacity Utilization		14.0%	IC	U Level	of Service			Α				
Analysis Period (min)			15									
,												

	<b>→</b>	•	1	•	1	-
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>†</b>			<b>^</b>	7	7
Traffic Volume (veh/h)	342	0	0	675	46	38
Future Volume (Veh/h)	342	0	0	675	46	38
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	446	0	0	880	60	50
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume			446		886	446
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			446		886	446
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		79	91
cM capacity (veh/h)			1111		284	560
	ED 4	WD 4		ND 4		
Direction, Lane #	EB 1	WB 1	WB 2	NB 1	NB 2	
Volume Total	446	440	440	60	50	
Volume Left	0	0	0	60	0	
Volume Right	0	0	0	0	50	
cSH	1700	1700	1700	284	560	
Volume to Capacity	0.26	0.26	0.26	0.21	0.09	
Queue Length 95th (m)	0.0	0.0	0.0	5.9	2.2	
Control Delay (s)	0.0	0.0	0.0	21.0	12.1	
Lane LOS				С	В	
Approach Delay (s)	0.0	0.0		17.0		
Approach LOS				С		
Intersection Summary						
Average Delay			1.3			
Intersection Capacity Utiliza	ation		32.4%	IC	U Level c	f Service
Analysis Period (min)			15			22

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Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	*	<b>†</b>			7	
Traffic Volume (veh/h)	34	45	0	0	22	0
Future Volume (Veh/h)	34	45	0	0	22	0
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	44	59	0	0	29	0
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	0				147	0
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	0				147	0
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	97				96	100
cM capacity (veh/h)	1623				822	1085
Direction, Lane #	EB 1	EB 2	SB 1			
Volume Total	44	59	29			
Volume Left	44	0	29			
Volume Right	0	0	0			
cSH	1623	1700	822			
Volume to Capacity	0.03	0.03	0.04			
Queue Length 95th (m)	0.03	0.03	0.04			
Control Delay (s)	7.3	0.0	9.5			
Lane LOS		0.0				
	A 3.1		A 9.5			
Approach LOS	ა. I					
Approach LOS			Α			
Intersection Summary						
Average Delay			4.5			
Intersection Capacity Utiliza	ition		13.3%	IC	U Level c	of Service
Analysis Period (min)			15			

